A Framework for Evaluating Deterioration in Mobility and Resulting Economic Losses Due to Seismic Damage in Transportation Networks

+ work in progress

by

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A Framework for Evaluating Deterioration in Mobility and Resulting Economic Losses Due to Seismic Damage in Transportation Networks

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collaborators

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A Framework for Evaluating Deterioration in Mobility and Resulting Economic Losses Due to Seismic Damage in Transportation Networks

+ work in progress

sponsors









A Vision for Regional PBSA of Transportation Networks

+ work in progress



Outline

- Motivation and objectives (big picture)
- Vision and scope
- Details of envisioned components
- Some preliminary results and outlook



Big Picture



Why regional assessment?

- Hazards affect regions. The big picture is needed for

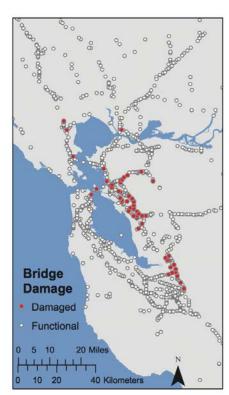
- Actuarial plans (insurance companies)
- Urban planning & public policy (government)
- Emergency service planning (1st responders)
- Built environment is highly interconnected
 - Residential neighborhoods, business centers
 - Transportation networks
 - Lifelines (water, power, communications)



Challenges

- Data
 metadata
 models
 - Diverse sample population (requires sophisticated—and as of yet non-existent—data harvesting tools)
 - Access to detailed data may be not be possible (requires estimation missing data, machine learning)
 - Processing requires *large* computational resources (would break records for civil engineers)
- Models decision variables
 - Heterogeneous analysis tools (OpenSees, OpenSHA, PACT)
 - New tech needs to be brought in (data analytics, Bayesian inference, etc.)

Objectives





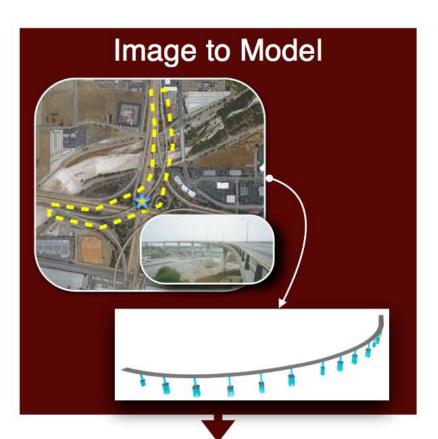
Risk framework for a highway network (Miller & Baker, 2015)

Develop a (semi-) automated interactive platform that can evaluate seismic vulnerability of complex transportation networks:

- 1. Generate structural models using data harvested from various sources
- 2. Carry out site- and structurespecific seismic analyses
- 3. Evaluate the consequent economic losses at the network-level



Vision and Scope





Decision Variables

- Losses

- etc.

DowntimeRepair CostRetrofit CostInsurance







structural system and nonstructural components, loss estimates

meta-data-bases for

















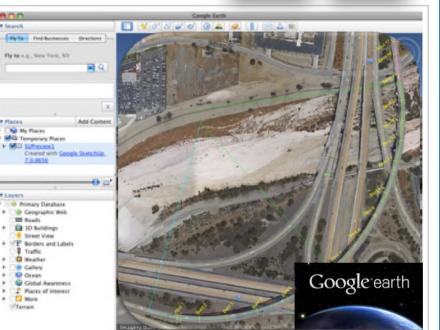




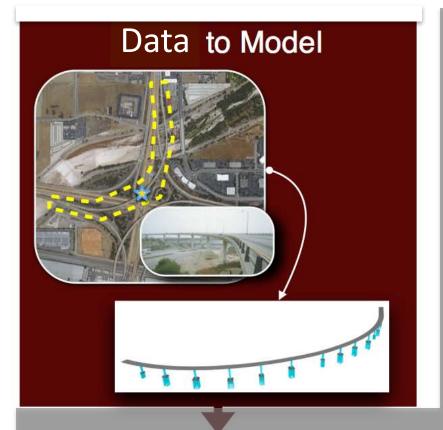
1 / E O C O U P B & N O F

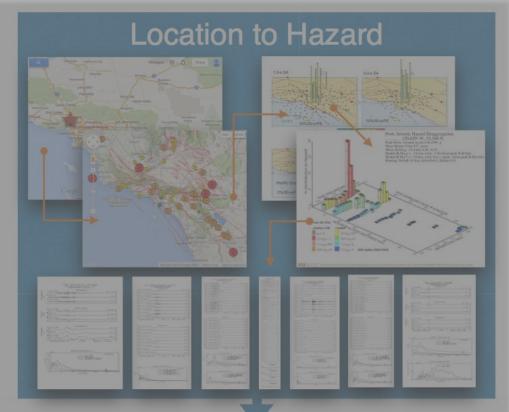




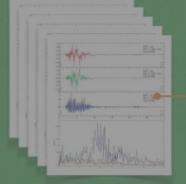


Details of the Envisioned Components

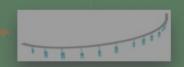




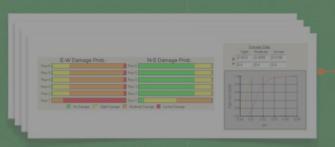
Analysis to Decision



seismic loads



analysis model



fragility curves

Decision Variables

- Losses
- Downtime
- Repair Cost
- Retrofit Cost
- Insurance
- etc

Where is the data coming from?

National Bridge Inventory (NBI) by FHWA

Caltrans Bridge Database

California Strong Motion Instrumentation Program (CSMIP)

Database





Number of Spans	20
Plan Shape	Straight
Total Length	2507" (764.1m)
Width of Deck	34' (10.4m)
Construction Year	1971
Instrumentation Year	1996
Seismic Retrofit	2006









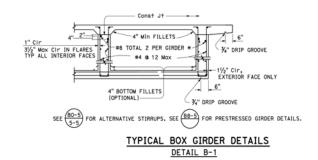


Where is the data coming from?

Guideline Documents

Caltrans Standard Plans

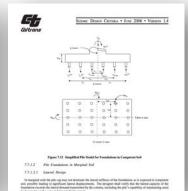
allow determination of many metadata elements (e.g., abutment seat length, shear-key reinforcement, foundation configuration, etc.)



Caltrans Seismic Design Criteria
 Manual (Caltrans SDC)

provides era-specific information on component and system design





Aggregation studies

provides era-specific structural configuration, probability distributions of structural properties

(Mangalathu, 2017)

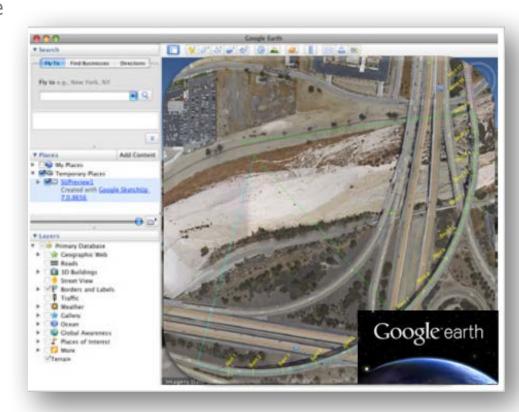


Where is the data coming from?

Internet Harvesting

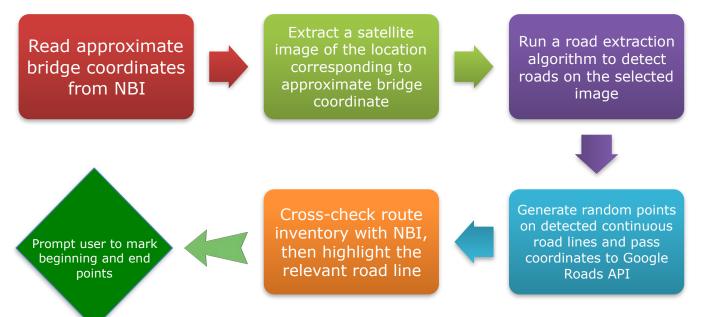
Google Maps/Earth, MapQuest, etc.

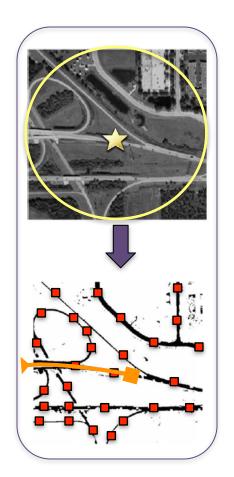
can be interrogated online more on this later ...





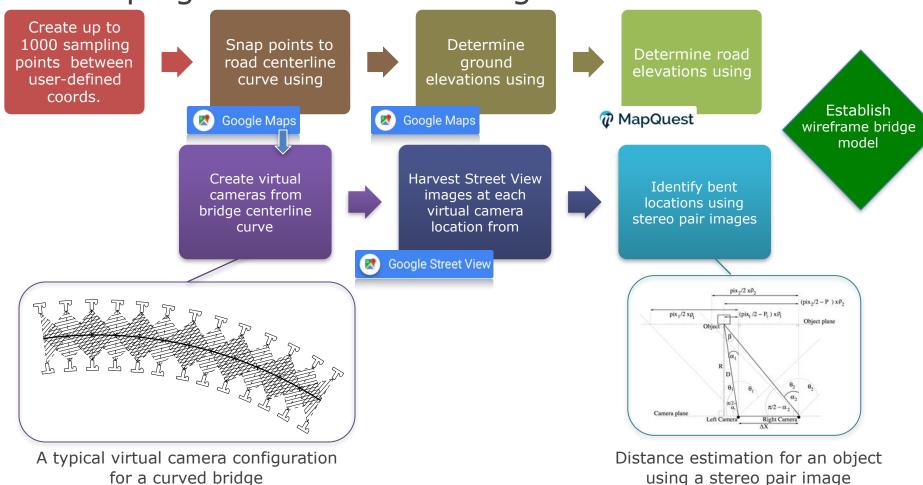
Detection of Bridge Locations







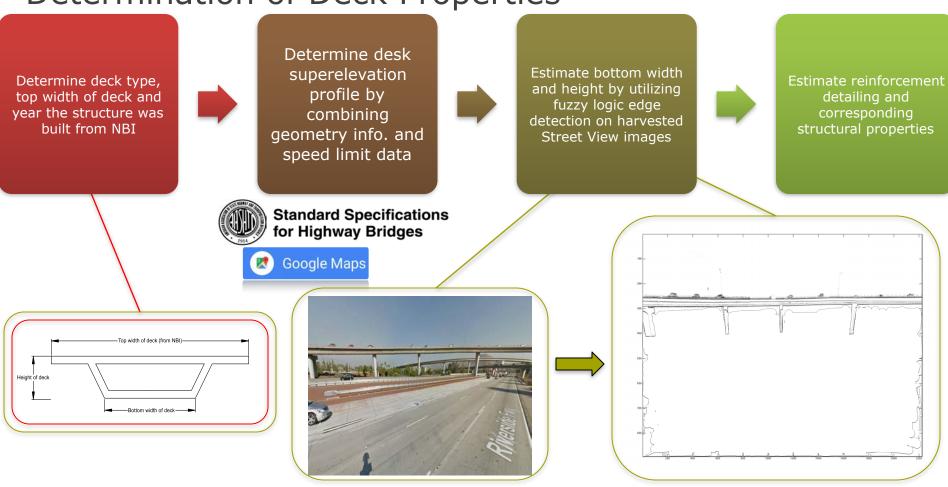
Developing the Wireframe Bridge Models





using a stereo pair image

Determination of Deck Properties





Determination of Column Properties

21.2.1.2 Column Reinforcement Requirements

(1) Longitudinal Reinforcement

Maximum Longitudinal Reinforcement Area, $A_{st,max} = 0.04 \times A_g$ (SDC 3.7.1-1) Minimum Longitudinal Reinforcement Area:

$$A_{st,min} = 0.0 \, l(A_g)$$
 for columns (SDC 3.7.2-1)
 $A_{st,min} = 0.005(A_g)$ for Pier walls (SDC 3.7.2-2)

where:

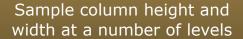
 A_g = the gross cross sectional area (in.²)

Normally, choosing column $A_{st} = 0.015(A_g)$ is a good starting point.

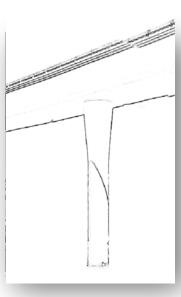
(2) Transverse Reinforcement

Either spirals or hoops can be used as transverse reinforcement in the column. However, hoops are preferred (see MTD 20-9) because of their discrete nature in the case of local failure.

Determine the column type based on the number of detected column edges

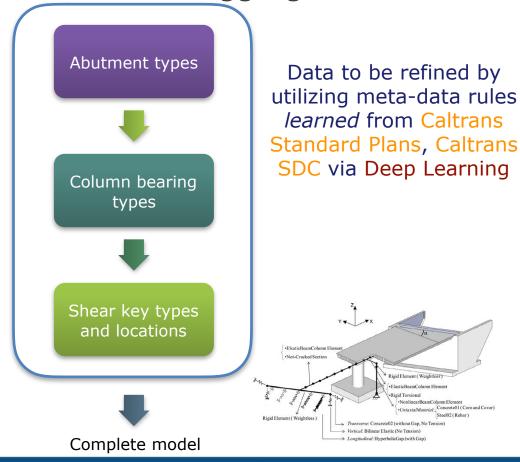


Estimate rebar detailing and corresponding structural properties by interrogating a database of similar columns (and by utilizing Caltrans SDC)

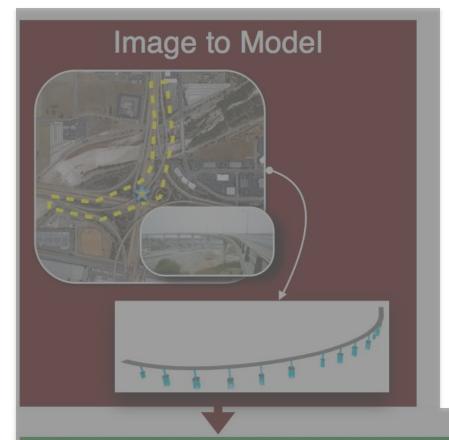




Completion of model using metadata harvested from the databases and estimates from aggregation studies

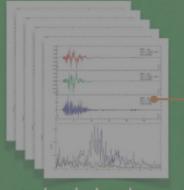




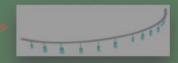




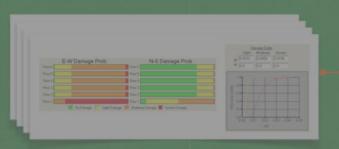
Analysis to Decision



seismic loads



analysis model



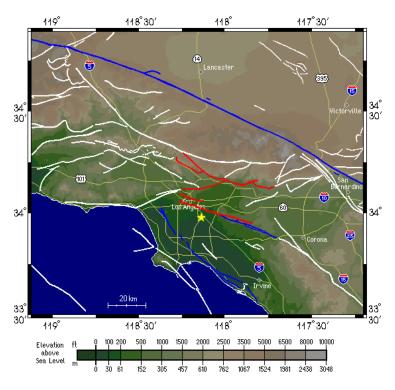
fragility curves

Decision Variables

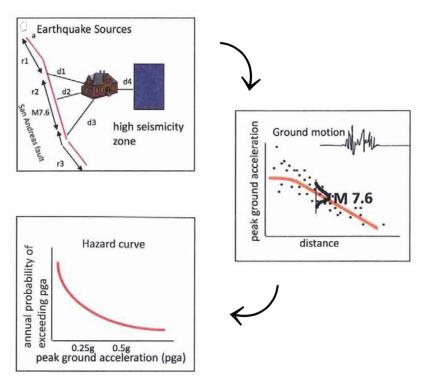
- Losses
- Downtime
- Repair Cost
- Retrofit Cost
- Insurance
- etc

Location to Hazard

Probabilistic Seismic Hazard Assessment (PSHA)



A map of active faults around a Los Angeles site (Stewart, 2014)



Basic seismic hazard methodology (from Boore et al.)

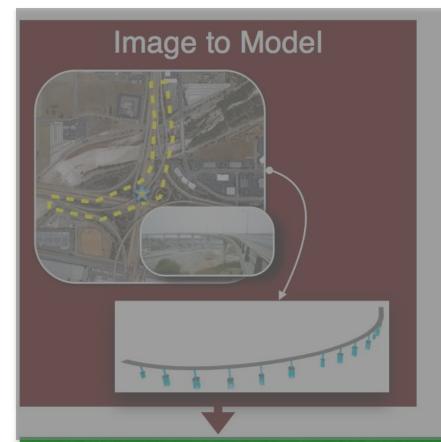


PEER

PACIFIC EARTHQUAKE ENGINEERING RESEARCH CENTER

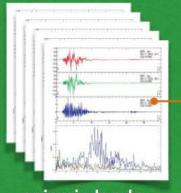








Analysis to Decision



seismic loads



analysis model



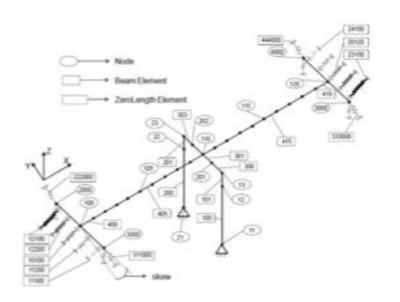
fragility curves

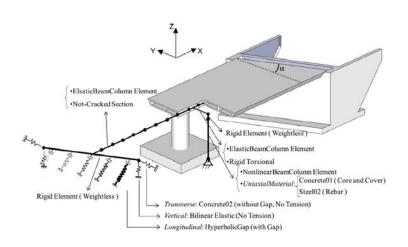
Decision Variables

- Losses
- Downtime
- Repair Cost
- Retrofit Cost
- Insurance
- etc.

Analysis Models

Building blocks of a bridge model





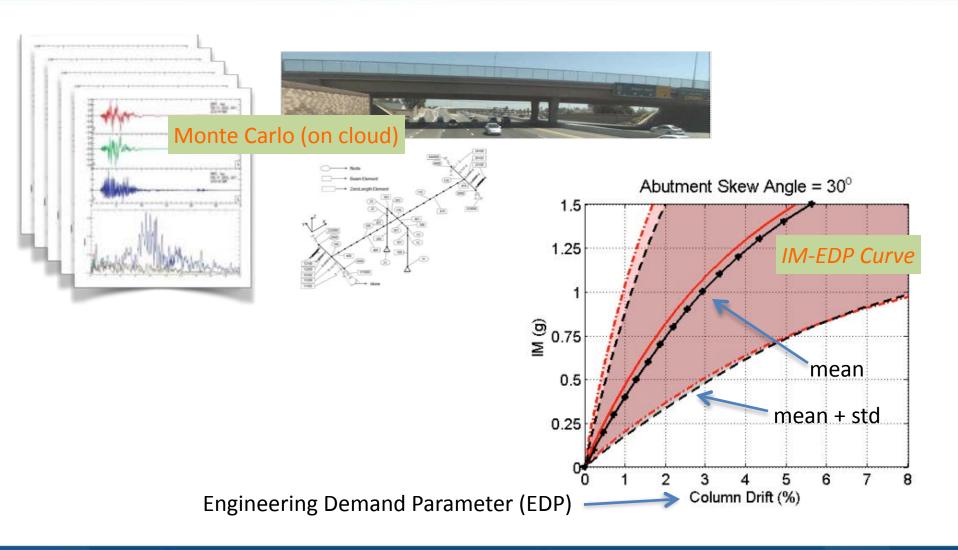
- Piles [Boulanger et al., 1999; Taciroglu et al., 2006; Khalili-Tehrani et al., 2014]
- Abutments [Stewart et al. 2007; Shamsabadi et al., 2010; Nojoumi et al., 2015]
- Shear keys [Mobasher et al., 2015; Omrani et al., 2015]
- In-span hinges [Trochalakis et al., 1997; Hube and Mosalam, 2008]
- Columns [Barry and Eberhard, 2008]
- Girders, deck (elastic)

Detailed descriptions of component and system modeling are provided in

Omrani R, Mobasher B, Liang X, Gunay S, Mosalam K, Zareian F, Taciroglu E (2015). *Guidelines for Nonlinear Seismic Analysis of Ordinary Bridges: Version 2.0*, Caltrans Report No. 15-65A0454, Sacramento CA.

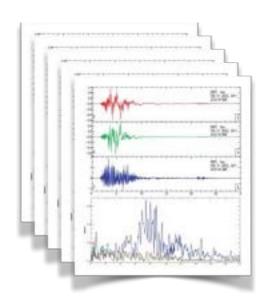


Analysis yields ...

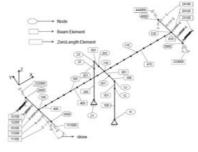




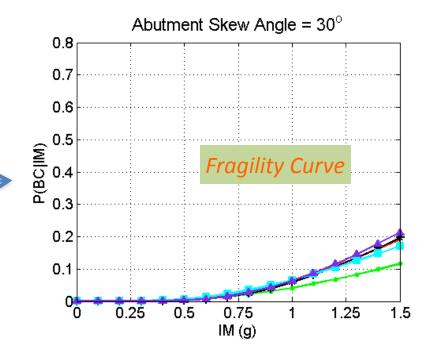
Analysis yields ...







Probability of Collapse (Collapse, Extensive, Moderate, Minor)



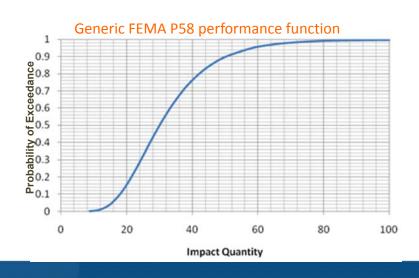


Loss & Recovery Estimation

an open problem for bridges

EDP or Performance State to Loss & Downtime

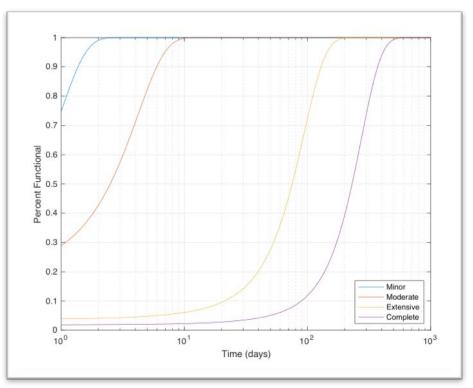
- Damage to a bridge leads to casualties and functional loss
 - Direct losses (repair cost) and indirect losses (downtime and casualties)
- Extensive research had been carried out for buildings
 - EDP to direct and indirect Losses (e.g., Porter, 2007; Mitrani-Reiser, 2007)
 - Packaged into FEMA Performance Assessment Calculation Tool (PACT)
 - Provides fragilities/performance-functions for structural and non-structural components, and systems





EDP or Performance State to Loss & Downtime

- Similar capabilities in loss estimation for bridges are lacking
- We currently use ATC-13 Bridge Restoration Curves





A Validation Study

San Bernardino – I-10/I-215 Interchange Bridge Coronado Bridge, San Diego CA

Validation study

San Bernardino – I-10/I-215 Interchange Bridge





Validation study

Selection of random points on the bridge by the user





Validation study

Initial processing of selected points by program



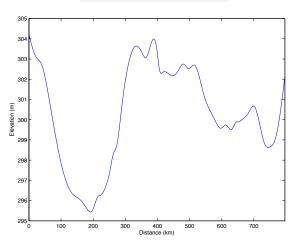
Calculation of bridge centerline curve

*Using **UCLA** automated image-based structural model development program through utilization

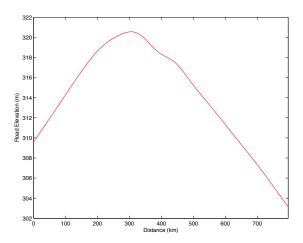
Google Maps Roads API

*Using **UCLA** automated image-based structural model development program through utilization of





Determination of ground elevations



Determination of road elevations

*Using **UCLA** automated image-based structural model development program through utilization of









Image processing to identify bent locations and developing the wireframe model

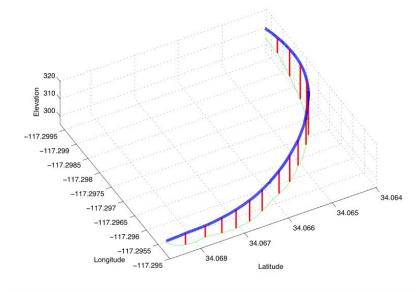


Identification of bent locations

*Using **UCLA** automated image-based structural model development program via *Image Analyzer Module*



*Using *UCLA* automated image-based structural model development program via *Wireframe Model Builder Module*



Establishing of wireframe model



Image processing to identify in-span hinge locations



Identification of in-span hinge locations

*Using \it{UCLA} automated image-based structural model development program via $\it{Image Analyzer Module}$



Using of auxiliary data to determine superelevation profile*



Determination of curve superelevation at each sampling point

Using **UCLA automated image-based structural model development program via *Image Analyzer Module* Identify centerline geometry in terms of constituent curves/spirals.

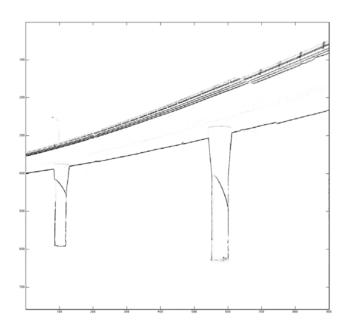
Get bridge speed limit data through Google Roads API.

Estimate curve superelevation at each sampling point.

Basic methodology to determine curve superelevation profile



Determination of bridge column dimensions



Detection of column edges

*Using **UCLA** automated image-based structural model development program via *Fuzzy Logic Edge Detection Module*



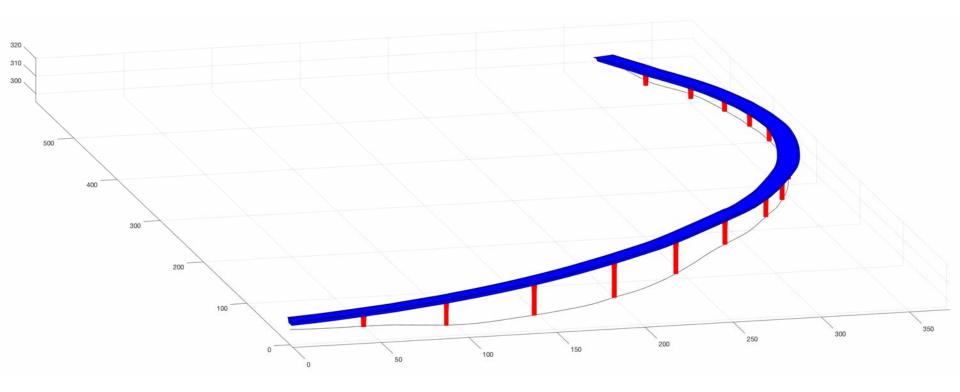
*Using **UCLA** automated image-based structural model development program via Pixel Counter Module



Determination of column dimensions



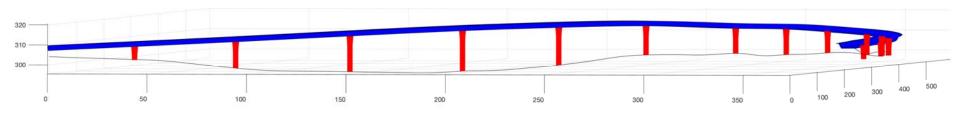
Resulting model





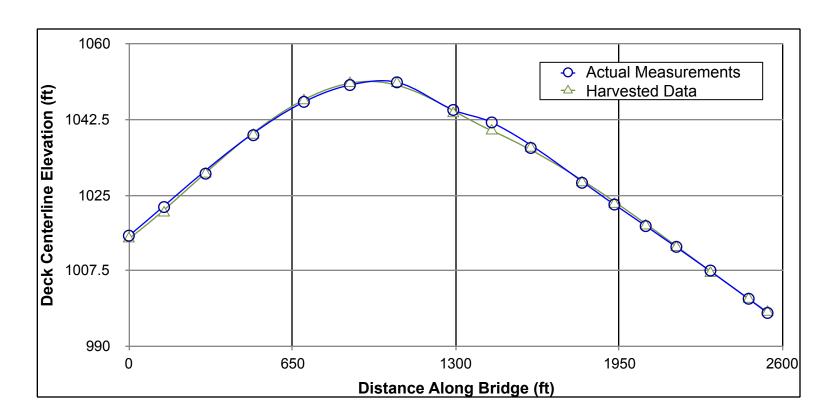
Resulting model





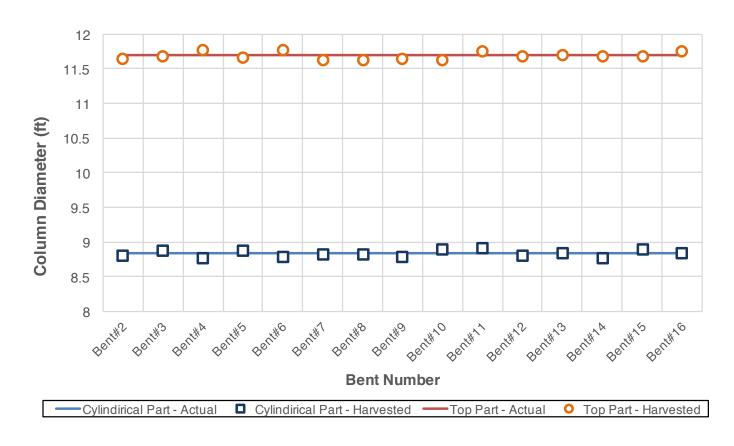


harvested data vs. as-built: bridge deck elevation



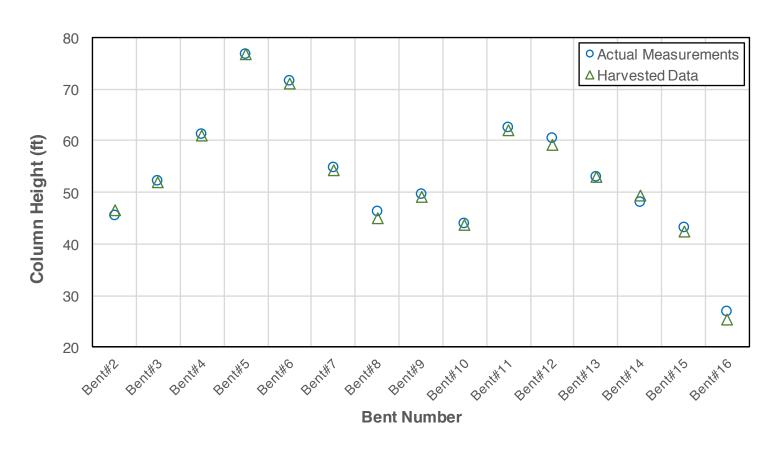


harvested data vs. as-built: column diameters





harvested data vs. as-built: column heights



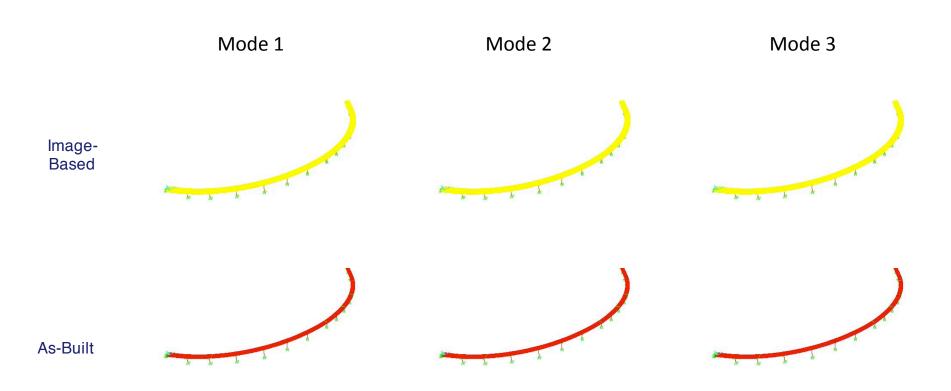


harvested data vs. as-built: modal periods

	T _{Image-Based} (sec)	T _{As-Built} (sec)
Mode 1	1.357	1.528
Mode 2	1.182	1.294
Mode 3	1.028	1.091
Mode 4	0.947	1.019
Mode 5	0.892	0.942
Mode 6	0.836	0.881
Mode 7	0.784	0.807
Mode 8	0.746	0.788



harvested data vs. as-built: mode shapes

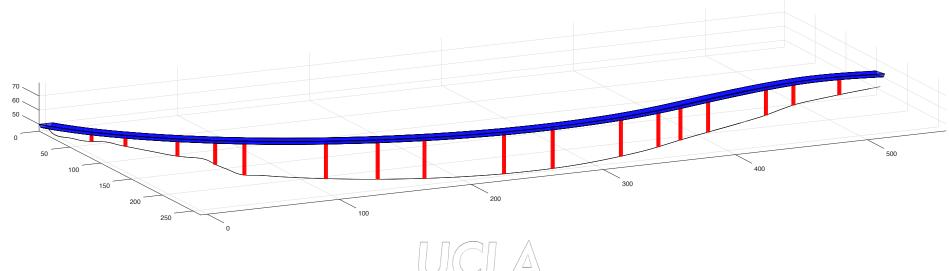




Other Examples

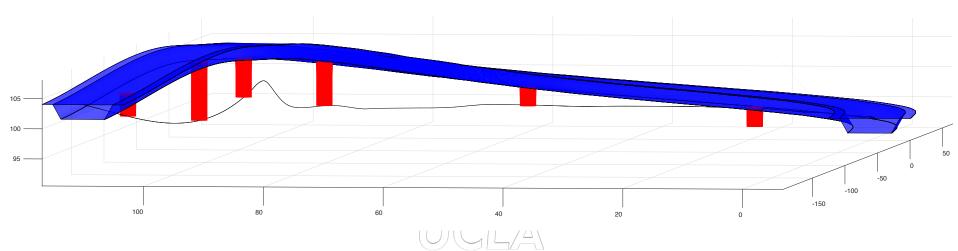
Sample Application: LA I10/I405N Interchange





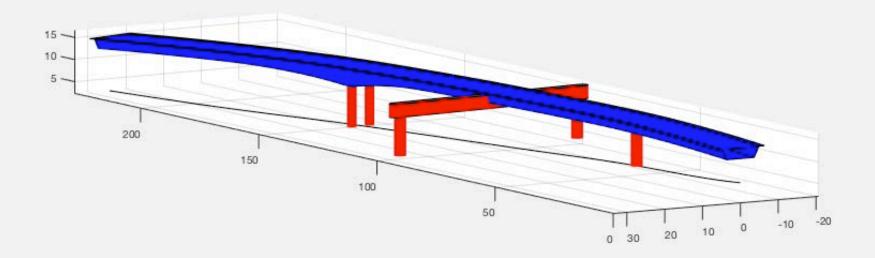
Sample Application: LA Wilshire/I-405N On-Ramp





Sample Application: LA I405N/CA22W Interchange





Regional Assessment Application Example

Port of Los Angeles

Region of Interest

Port of Los Angeles

- 5x6 miles rectangular region containing all critical bridges connecting to Port of Los Angeles
- ROI contains 95 bridges
- 62 bridges built <1970 and have not been retrofitted





Bridge Model Inventory

- Geometries generated with UCLA tool from street view + satellite images
- Structural properties are assigned probabilistically based on a Georg Tech study of California bridges (Mangalathu, 2017)
- The study contains statistical distributions for
 - Concrete compressive strength*
 - Steel yield strength*
 - Longitudinal steel reinforcement ratio
 - Transverse steel reinforcement ratio*
 - Foundation translation and rotational stiffness*
 - Damping
 - Mass factor

mean values here

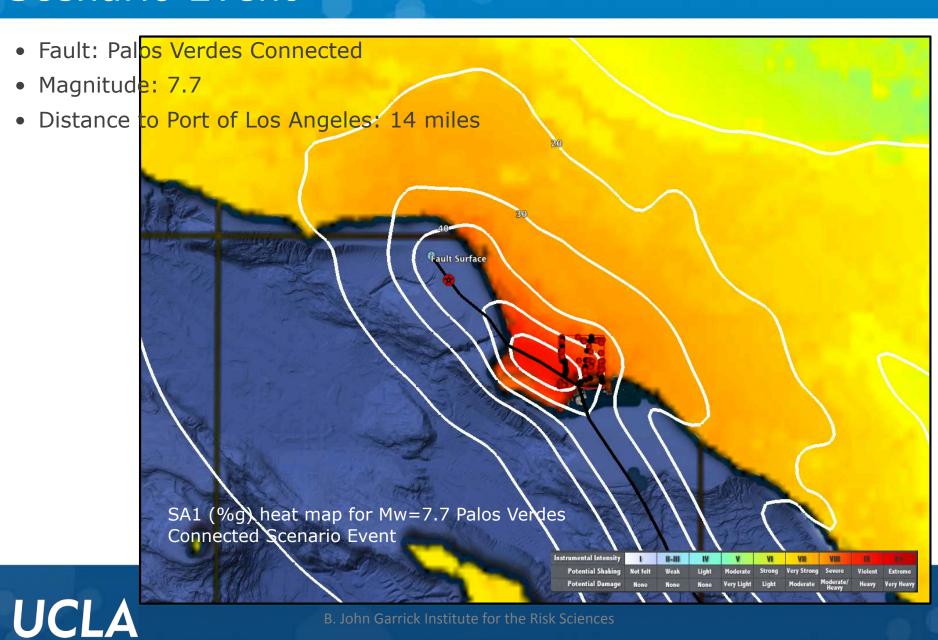
	Distribution					
Design Era	Туре	Mean	Standard Deviation	Lower Bound	Upper Bound	
Era 1	#4 @ 12 in. irrespective of the cross-section					
Era 2/ Era 3	Uniform	0.85	0.07	0.4	0.14	





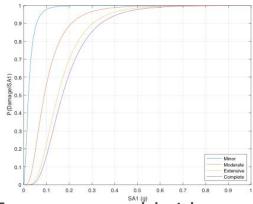


Scenario Event

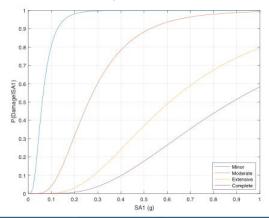


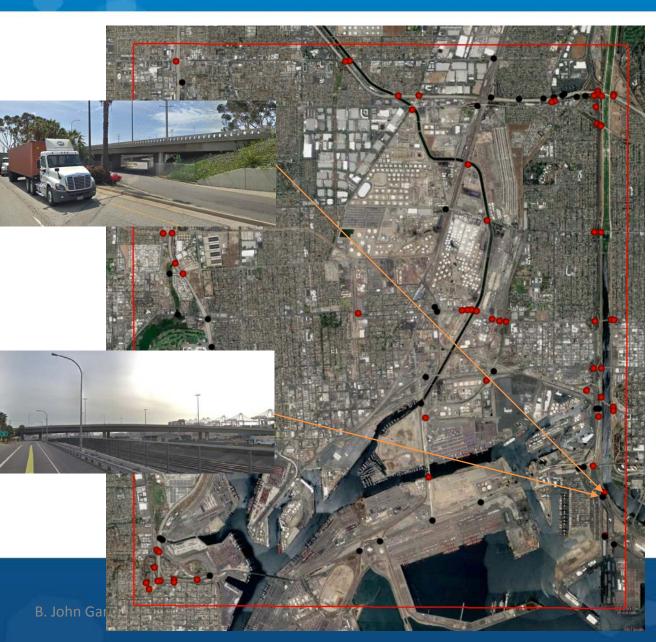
Bridge Closures





5-span curved bridge YB=1997, seat abutment







Quo Vadis?

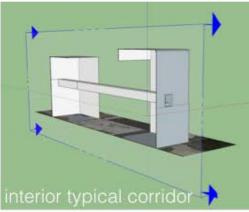
- Further develop Image to Model Capabilities
- Develop user-interface (a GIS-integrated web site)
- Combine bridge closure data with traffic congestion simulation and estimate economic losses (USC collaboration)
- Expand to Region of Interest
- Consider realistic aftershock effects

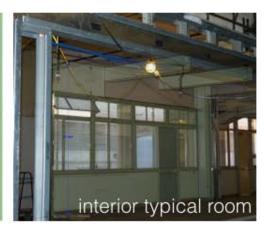


What about Buildings?

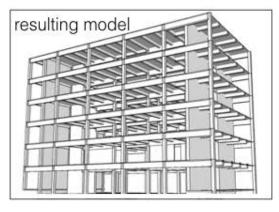
Building models from image data









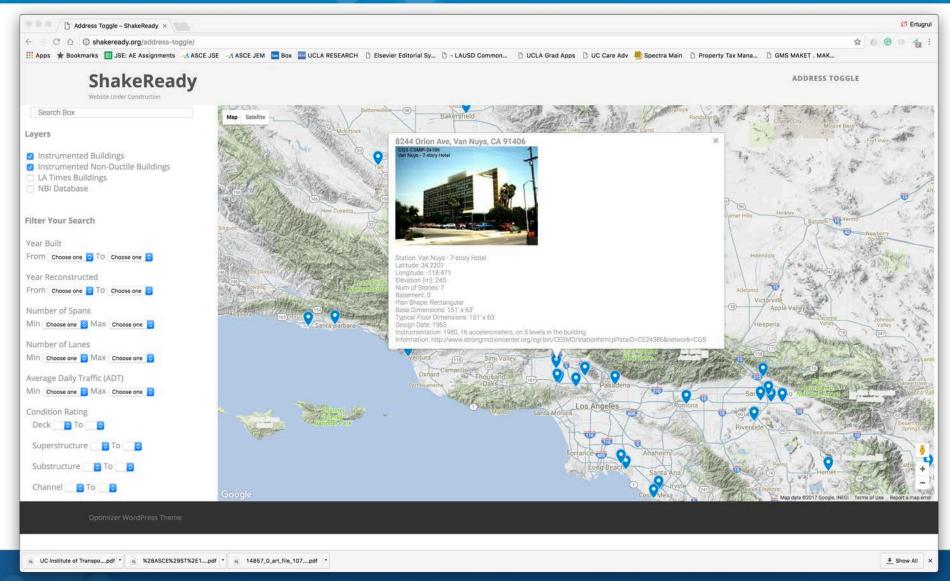




ShakeReady

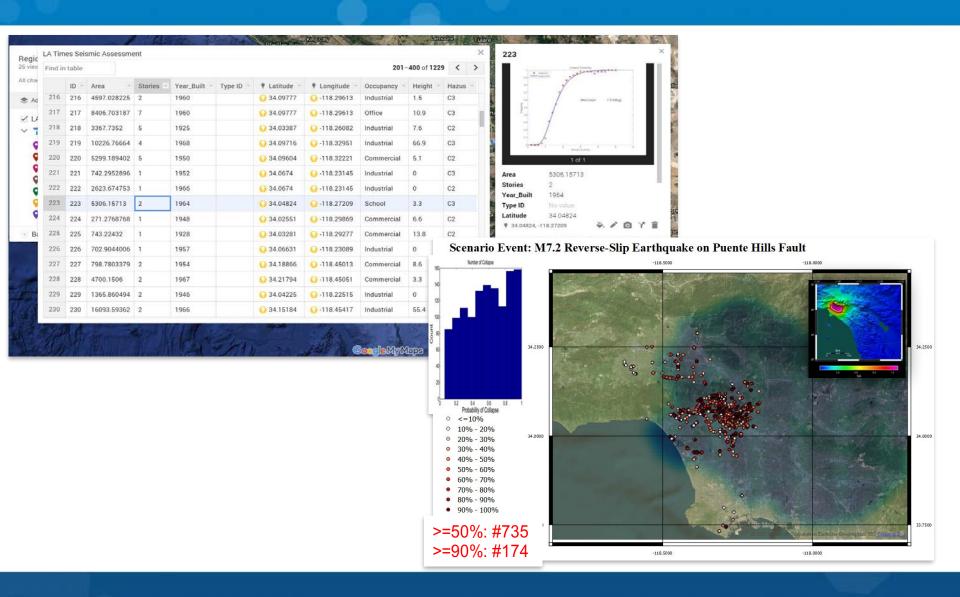
a user interface under development

Building inventories





Non-ductile reinforced concrete buildings under a scenario event





thank you! etacir@ucla.edu

